

Contents lists available at SCCE

Journal of Soft Computing in Civil Engineering

Journal homepage: www.jsoftcivil.com



An Equation to Determine the Ultimate Flexural Load of RC Beams Strengthened with CFRP Laminates

R. Farahnaki^{1*}, A. Azimi²

 The University of Wollongong, Wollongong, Australia
 Department of Civil Engineering, University of Birmingham, Birmingham, United Kingdom Corresponding author: rf847@uowmail.edu.au

di https://doi.org/10.22115/SCCE.2018.136500.1076

ARTICLE INFO

Article history: Received: 19 June 2018 Revised: 29 September 2018 Accepted: 31 October 2018

Keywords: Concrete beam; Flexural load; FRP; Strengthening.

ABSTRACT

In this paper, a new relationship is presented for determining the ultimate flexural load of reinforced concrete beams strengthened with CFRP laminates. An artificial neural network with a suitable performance was used to estimate this equation. First, a collection of laboratory results including 83 data was collected from valid references. This database was then divided into three groups of 51, 16, and 16, which were used to train, validation, and test the proposed equation, respectively. The final model had eleven inputs including concrete compressive strength, width of beam, effective depth, area of tension reinforcement, area of compression reinforcement, yield strength of steel, modulus of elasticity of steel, modulus of elasticity of CFRP sheet, width of CFRP sheet, total thickness of CFRP sheets and, length of CFRP sheet, which were applied to the network to determine the ultimate flexural load as the output of the model. The obtained results from the proposed relationship showed that it was able to use as a predictive equation for the considered target.

1. Introduction

Today, FRP materials are widely used for retrofitting. FRP materials can use in three types including layers, grainy material and also bars. The retrofitting effects of FRP plate instead of bars was studied by researchers [1]. Some researchers developed the theoretical approaches to study the flexural capacity of FRP concrete beams [2]. Previous studies have shown that the How to cite this article: Farahnaki R, Azimi A. An equation to determine the ultimate flexural load of RC beams strengthened with CFRP laminates. J Soft Comput Civ Eng 2018;2(4):86–95. https://doi.org/10.22115/scce.2018.136500.1076.

This is an open access article under the CC BY license (http://creativecommons.org/licenses/by/4.0/).



performance of FRP materials is suitable for increasing the capacity of structural elements and for this reason, their use in the building industry is expanding. Finite elements method, which is considered as a suitable tool for analyzing the structures [3] and determining the forces of elements, was investigated by Qu *et al.* [4] to estimate failure modes of FRP strengthened concrete beams. In another study, Xu [5] simulated FRP concrete beams by finite elements method and investigated the bearing capacity of RC beams. The moment capacity of RC beams strengthened with FRP based artificial neural networks was predicted by Zatloukal and Konvalinka [6].

Soft computing methods, which are very efficient models, are used for engineering problems. Their application in various fields of civil engineering has also been evaluated by researchers [7–20]. In this paper, neural networks as a soft computing method are considered to propose a new capable equation to predict the ultimate flexural load in reinforced concrete beams strengthened with CFRP laminates. A collection of experimental results is used to train and validate the equation. Details of the method are described and illustrated in the next sections.

2. Definition of the database and selected parameters

One of the most important factors in the construction of a neural network is a valid database, which is used to train the network and determine its unknown parameters. In this paper, the authors used the experimental results of RC beams strengthened with FRP that carried out by researchers [21-37]. This database includes 83 data that has been used in three groups of 51, 16 and 16 for training, evaluating and also testing of the proposed model.

In addition, eleven parameters were used as inputs in the presented neural network including concrete compressive strength, width of beam, effective depth, area of tension reinforcement, area of compression reinforcement, yield strength of steel, modulus of elasticity of steel, modulus of elasticity of CFRP sheet, width of CFRP sheet, total thickness of CFRP sheets and also length of CFRP sheet. Details of these parameters are shown in Table 1 and also Fig.1. Based on the database collected for this article, the amounts of maximum and minimum of each parameter were reported in the table.

Variable	Description	Minimum	Maximum
<i>x</i> ₁	Concrete compressive strength, MPa	18	55.2
x_2	Width of beam, mm	100	500
x_3	Effective depth, mm	50.8	419
x_4	Area of tension reinforcement, mm ²	71	2413
x_5	Area of compression reinforcement, mm ²	28	1609
x_6	Yield strength of steel, MPa	335	590
x_7	Modulus of elasticity of steel, GPa	165	201
x_8	Modulus of elasticity of CFRP sheet, GPa	11	240
x_9	Width of CFRP sheet, mm	25	480
x_{10}	Total thickness of CFRP sheets, mm	0.111	6
x_{11}	Length of CFRP sheet, mm	1200	4800
Y	Ultimate flexural load, kN	16.1	669.3

Table 1

The considered parameters for the proposed model.



Fig. 1. Details of the selected parameters

In modeling of neural networks, two processes are used as preprocesses of data, which include changing the ordering of data or randomizing, as well as normalizing data or reducing the range of parameter variations. In this paper, the authors used Eq.1 to normalize the database. This relationship limits the range of each of parameters between -1 to +1.

$$X_{i} = 2\frac{x_{i} - x_{mi}}{x_{ma} - x_{mi}} - 1$$
(1)

In this equation, parameters X_i, x_i, x_{mi}, x_{ma} are normal, experimental, minimum and also maximum value of the variables. Based on this equation, the normal values of input parameters can calculate by Eqs.2.

$$X_{1} = 2\frac{x_{1} - 18}{37.2} - 1 \qquad X_{2} = 2\frac{x_{2} - 100}{400} - 1 \qquad X_{3} = 2\frac{x_{3} - 50.8}{368.2} - 1 \qquad X_{4} = 2\frac{x_{4} - 71}{2342} - 1$$
$$X_{5} = 2\frac{x_{5} - 28}{1581} - 1 \qquad X_{6} = 2\frac{x_{6} - 335}{255} - 1 \qquad X_{7} = 2\frac{x_{7} - 165}{36} - 1 \qquad X_{8} = 2\frac{x_{8} - 11}{229} - 1 \qquad (2)$$
$$X_{9} = 2\frac{x_{9} - 25}{455} - 1 \qquad X_{10} = 2\frac{x_{10} - 0.111}{5.889} - 1 \qquad X_{11} = 2\frac{x_{11} - 01200}{3600} - 1$$

3. Predictive equation

To determine the predictive equation in this paper, the authors used the neural network with a single neuron and one hidden layer. Fig.2 shows the general structure of the mentioned neural network. The parameters W and b in this figure are related to weight and bias. The function considered for the node of the hidden layer in this network is a Sigmoid logarithmic function.



Fig. 2. The structure of the neural network.

Train data is used to train the network and determine the weights and biases of the neural network structure. The results of the training phase are shown in Fig.3 and Fig.4.



Fig. 4. Results of the proposed network for training data.

After training the network with 51 data intended for training and 16 data for evaluating, the unknown parameters of the network were determined as follows, in which, IW, LW, b1 and b2 were input weights, layer weight, the bias of the hidden layer and also the bias of the output layer.

$$[IW] = \begin{bmatrix} -0.0831 & 0.0974 & -0.4439 & -0.9075 & 0.1203 & -0.1085 & -0.0071 & 0.0412 & -0.3204 & 0.0809 & 0.1325 \end{bmatrix}$$

 $LW = -3.6649$
 $b_1 = 0.3598$
 $b_2 = 2.0099$

To determine the output of the hidden layer, the weight of each input should apply to its corresponding input value. Then, the sum of the resulting values with adding the hidden layer bias (Eq.3) is applied to Sigmoid logarithm function (Eq.4).

$$\begin{bmatrix} X \end{bmatrix} = \begin{bmatrix} X_1 & X_2 & X_3 & X_4 & X_5 & X_6 & X_7 & X_8 & X_9 & X_{10} & X_{11} \end{bmatrix}$$

$$Y_1 = \begin{bmatrix} IW \end{bmatrix} \begin{bmatrix} X \end{bmatrix}^T + b_1$$

$$= -0.0831X_1 + 0.0974X_2 - 0.4439X_3 - 0.9075X_4 + 0.1203X_5 - 0.1085X_6 - 0.0071X_7 \qquad (3)$$

$$+ 0.0412X_8 - 0.3204X_9 + 0.0809X_{10} + 0.1325X_{11} + 0.3598$$

$$Y_2 = -3.6649 \left(\frac{1}{1 + e^{-Y_1}}\right) + b_2 \qquad (4)$$

Based on the above equations and the obtained results of the coefficients, Eq.5 is proposed as the predictive model for determining the ultimate flexural load of reinforced concrete beams strengthened with CFRP laminates.

$$Y(kN) = 326.6(Y_2 + 1) + 16.1$$
(5)



As mentioned in the previous sections, to evaluate the proposed equation of this research, a collection of 16 datasets. The results of this datasets are presented in Fig. 5 and Fig. 6 which are indicated that the model had reasonable outputs against its corresponding targets.



4. Discussion

In this section, the ability of the Eq.5 is investigated by 16 datasets as test data. To do this, the outputs of this datasets were calculated and presented in Fig. 7 and Fig. 8. The figures indicated that the proposed equation was evaluated and tested with results of very close to the

experimental results. For all 83 laboratory data, Fig. 7 and Fig. 10 showed the outputs of the presented equation and indicated that the errors are poor and the correlation coefficient between predicted values and laboratory results is also very close. Therefore, the results obtained in this paper show that Eq.5 is suitable for estimating the selected output.



Fig. 9. Scatter plot for all data.

5. Conclusion

In this paper, a relationship is proposed for estimating the ultimate flexural load in reinforced concrete beams strengthened with CFRP laminates. The effective parameters on the target, which are used as inputs of the proposed model, are the concrete compressive strength, width of beam, effective depth, area of tension reinforcement, area of compression reinforcement, yield strength of steel, modulus of elasticity of steel, modulus of elasticity of CFRP sheet, width of CFRP sheet, total thickness of CFRP sheets and, length of CFRP sheet. The unknown coefficients of the proposed equation are driven using a neural network model as well as the results of the laboratory tests. Also, the proposed Equation has good accuracy, which indicates that this relationship can predict the considered output parameter.

References

- [1] Seo SY, Choi KB, Kwon YS. Retrofit Capacity of Near-Surface-Mounted RC Beam by using FRP Plate. J Korea Inst Struct Maint Insp 2012;16:18–26. doi:10.11112/jksmi.2012.16.1.018.
- [2] Dolan CW, Swanson D. Development of flexural capacity of a FRP prestressed beam with vertically distributed tendons. Compos Part B Eng 2002;33:1–6. doi:10.1016/S1359-8368(01)00053-1.
- [3] Kheyroddin A, Mirrashid M, Arshadi H. An Investigation on the Behavior of Concrete Cores in Suspended Tall Buildings. Iran J Sci Technol Trans Civ Eng 2017;41:383–8. doi:10.1007/s40996-017-0075-y.
- [4] Qu HC, Wu CQ, Chen LL. Numerical Analysis on the Load-Carrying Capacity for the FRP Reinforced Four-Point Bending Concrete Beam. Adv Mater Res 2011;287–290:1130–4. doi:10.4028/www.scientific.net/AMR.287-290.1130.

- [5] Xu X. Calculation Method and Analysis of Bearing Capacity of FRP Rebar Concrete Beam. ICTE 2011, Reston, VA: American Society of Civil Engineers; 2011, p. 1572–7. doi:10.1061/41184(419)260.
- [6] Zatloukal J, Konvalinka P. Moment Capacity of FRP Reinforced Concrete Beam Assessment Based on Centerline Geometry. Appl Mech Mater 2013;486:211–6. doi:10.4028/www.scientific.net/AMM.486.211.
- [7] Jafari M, Mirrashid M, Vahidnia A. Prediction of chloride penetration in the concrete containing magnetite aggregates by Adaptive Neural Fuzzy Inference System (ANFIS). 7th Internatinal Symp. Adv. Sci. Technol. (5thsastech), Bandare Abbas, Iran, 2013.
- [8] Mirrashid M. Earthquake magnitude prediction by adaptive neuro-fuzzy inference system (ANFIS) based on fuzzy C-means algorithm. Nat Hazards 2014;74:1577–93. doi:10.1007/s11069-014-1264-7.
- [9] Naderpour H, Mirrashid M. Shear Strength Prediction of RC Beams Using Adaptive Neuro-Fuzzy Inference System. Sci Iran 2018:0–0. doi:10.24200/sci.2018.50308.1624.
- [10] Naderpour H, Mirrashid M. An innovative approach for compressive strength estimation of mortars having calcium inosilicate minerals. J Build Eng 2018;19:205–15. doi:10.1016/j.jobe.2018.05.012.
- [11] Naderpour H, Rafiean AH, Fakharian P. Compressive strength prediction of environmentally friendly concrete using artificial neural networks. J Build Eng 2018;16:213–9. doi:10.1016/j.jobe.2018.01.007.
- [12] Naderpour H, Vahdani R, Mirrashid M. Soft Computing Research in Structural Control by Mass Damper (A review paper). 4st Int. Conf. Struct. Eng. Tehran, Iran, 2018.
- [13] Mirrashid M. Comparison Study of Soft Computing Approaches for Estimation of the Non-Ductile RC Joint Shear Strength. Soft Comput Civ Eng 2017;1:12–28. doi:10.22115/scce.2017.46318.
- [14] Mirrashid M, Bigdeli S. Genetic Algorithm for Prediction the Compressive Strength of Mortar Containing Wollastonite. 1st Natl. Congr. Counstruction Eng. Proj. Assessment, Gorgan, Iran, 2014.
- [15] Mirrashid M, Givehchi M, Miri M, Madandoust R. Performance investigation of neuro-fuzzy system for earthquake prediction. Asian J Civ Eng 2016;17:213–23.
- [16] Mirrashid M, Jafari M, Akhlaghi A, Vahidnia A. Prediction of compressive strength of concrete containing magnetite aggregates using Adaptive Neural Fuzzy Inference System (ANFIS). 4th Internatinal Conf. Concr. Dev., Tehran, Iran: 2013.
- [17] Naderpour H, Mirrashid M. Application of Soft Computing to Reinforced Concrete Beams Strengthened with Fibre Reinforced Polymers: A State-of-the-Art Review. Comput Tech Civ Struct Eng 2015;38:305–23.
- [18] Naderpour H, Mirrashid M. Compressive Strength of Mortars Admixed with Wollastonite and Microsilica. Mater Sci Forum 2017;890:415–8. doi:10.4028/www.scientific.net/MSF.890.415.
- [19] Naderpour H, Mirrashid M. Ultimate Capacity Prediction of Concrete Slabs Reinforced with FRP Bars. 3rd Int. 7th Natl. Conf. Mod. Mater. Struct. Civ. Eng. Bu-Ali Sina Univ. Hamedan, IRAN, 2018.
- [20] Naderpour H, Mirrashid M. Application of group method of data handling to Estimate the Shear Strength of RC Beams Reinforced with FRP Bars. 3rd Int. 7th Natl. Conf. Mod. Mater. Struct. Civ. Eng. Bu-Ali Sina Univ. Hamedan, IRAN, 2018.
- [21] HAJELA P, BERKE L. Neurobiological Computational Models in Structural Analysis and Design. 31st Struct. Struct. Dyn. Mater. Conf., Reston, Virigina: American Institute of Aeronautics and Astronautics; 1990. doi:10.2514/6.1990-1133.

- [22] Saadatmanesh H, Ehsani MR. RC Beams Strengthened with GFRP Plates. I: Experimental Study. J Struct Eng 1991;117:3417–33. doi:10.1061/(ASCE)0733-9445(1991)117:11(3417).
- [23] Lundqvist J, Nordin H, Täljsten B, Olofsson T. Numerical analysis of concrete beams strengthened with CFRP : a study of anchorage lengths. Int Symp Bond Behav FRP Struct 2005:239–46.
- [24] Maalej M, Leong KS. Effect of beam size and FRP thickness on interfacial shear stress concentration and failure mode of FRP-strengthened beams. Compos Sci Technol 2005;65:1148– 58. doi:10.1016/j.compscitech.2004.11.010.
- [25] Coronado CA, Lopez MM. Sensitivity analysis of reinforced concrete beams strengthened with FRP laminates. Cem Concr Compos 2006;28:102–14. doi:10.1016/j.cemconcomp.2005.07.005.
- [26] Reeve BZ. Effect of adhesive stiffness and CFRP geometry on the behavior of externally bonded CFRP retrofit measures subject to monotonic loads 2006.
- [27] Abdalla JA, Elsanosi A, Abdelwahab A. Modeling and simulation of shear resistance of R/C beams using artificial neural network. J Franklin Inst 2007;344:741–56. doi:10.1016/j.jfranklin.2005.12.005.
- [28] Esfahani MR, Kianoush MR, Tajari AR. Flexural behaviour of reinforced concrete beams strengthened by CFRP sheets. Eng Struct 2007;29:2428–44. doi:10.1016/j.engstruct.2006.12.008.
- [29] Neagoe CA. Concrete beams reinforced with CFRP laminates 2011.
- [30] C. Allen Ross Joseph W. Tedesco, and Mary L. Hughes DMJ. Strengthening of Reinforced Concrete Beams with Externally Bonded Composite Laminates. Struct J n.d.;96. doi:10.14359/612.
- [31] Ni H-G, Wang J-Z. Prediction of compressive strength of concrete by neural networks. Cem Concr Res 2000;30:1245–50. doi:10.1016/S0008-8846(00)00345-8.
- [32] Du Béton FI. Externally bonded FRP reinforcement for RC structures. Bulletin 2001;14:138.
- [33] Sanad A, Saka MP. Prediction of Ultimate Shear Strength of Reinforced-Concrete Deep Beams Using Neural Networks. J Struct Eng 2001;127:818–28. doi:10.1061/(ASCE)0733-9445(2001)127:7(818).
- [34] Dong Y, Zhao M, Ansari F. Failure characteristics of reinforced concrete beams repaired with CFRP composites. Strain 2002;304:12–7.
- [35] Dai JG, Ueda T, Sato Y, Ito T. Flexural strengthening of RC beams using externally bonded FRP sheets through flexible adhesive bonding. Int. Symp. Bond Behav. FRP Struct. Hong Kong, 2005, p. 7–9.
- [36] Ebead UA, Marzouk H. Tension-stiffening model for FRP-strenghened RC concrete two-way slabs. Mater Struct 2005;38:193–200. doi:10.1007/BF02479344.
- [37] Kotynia R. Debonding failures of RC beams strengthened with externally bonded strips. Proc. Int. Symp. Bond Behav. FRP Struct., 2005.